ABITA BREW PUB

EXTERIOR IMPROVEMENTS

72011 HOLLY ST.
ABITA SPRINGS, LA 70420

ISSUED FOR APPROVAL

ARCHITECT

CRUMPWILSON ARCHITECTS

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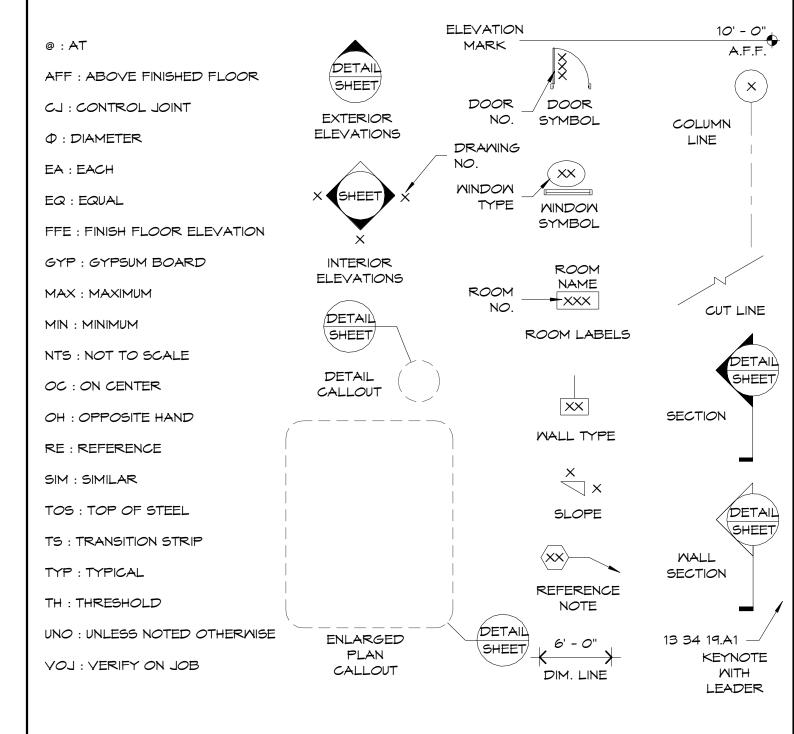
crumpwilsonarchitects.com

SITE MAP

N.T.S.



N.T.S.



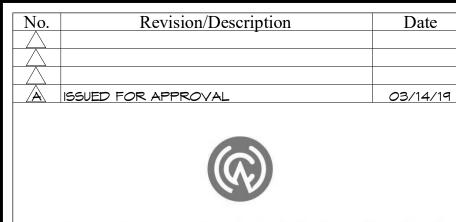
| SHEET INDEX

COVER TITLE & SHEET INDEX
DEMO DEMOLITION SITE PLAN
SITE PLAN

A100 FLOOR PLAN
A103 ROOF PLAN
A200 EXTERIOR ELEVATIONS
A201 BUILDING SECTION

REFERENCE NOTES

O1 PROJECT LOCATION



CRUMPWILSON

ABITA BREW PUB
EXTERIOR
IMPROVEMENTS

Drawing IIMIPKOVEIMIEN I

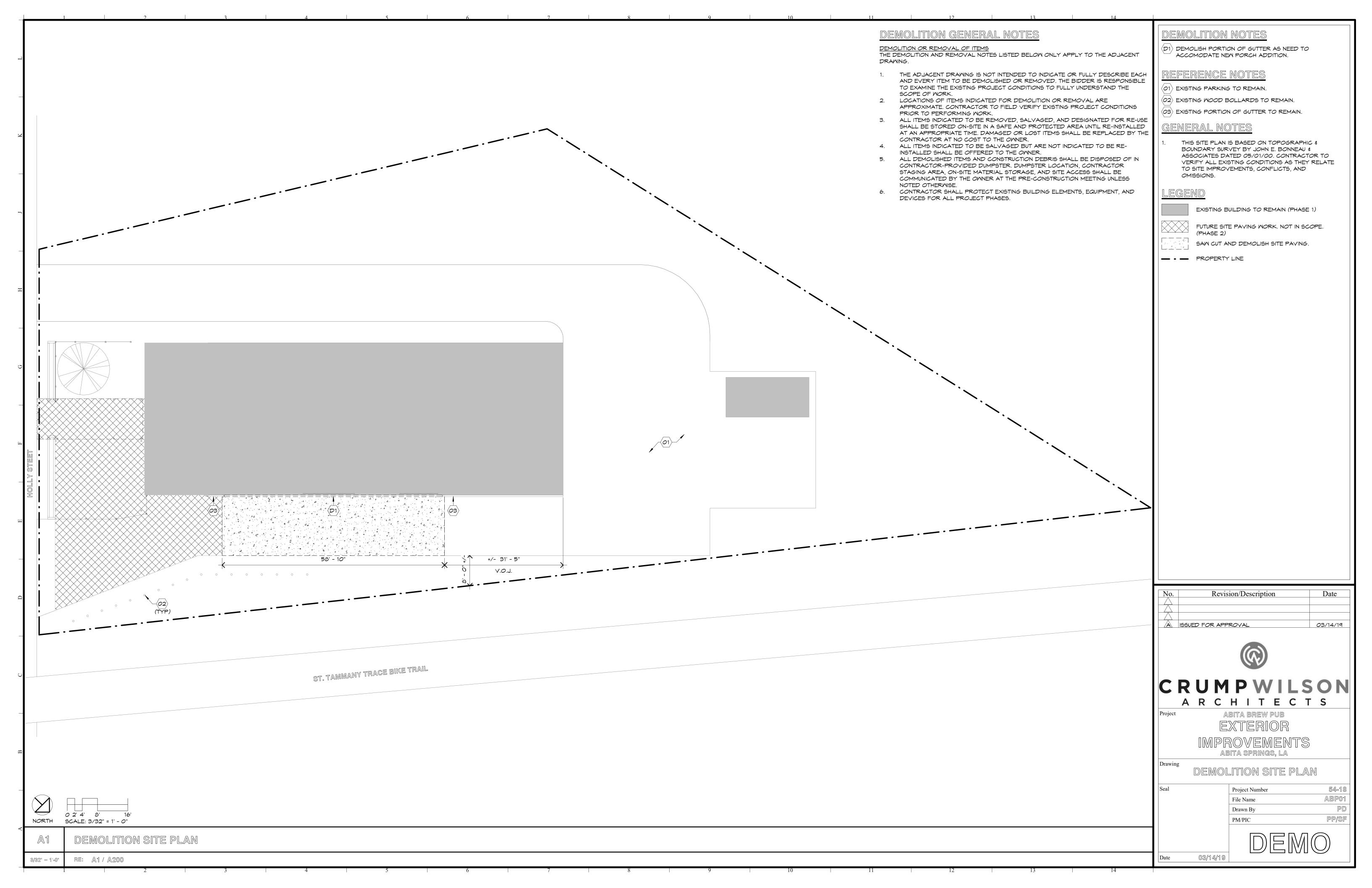
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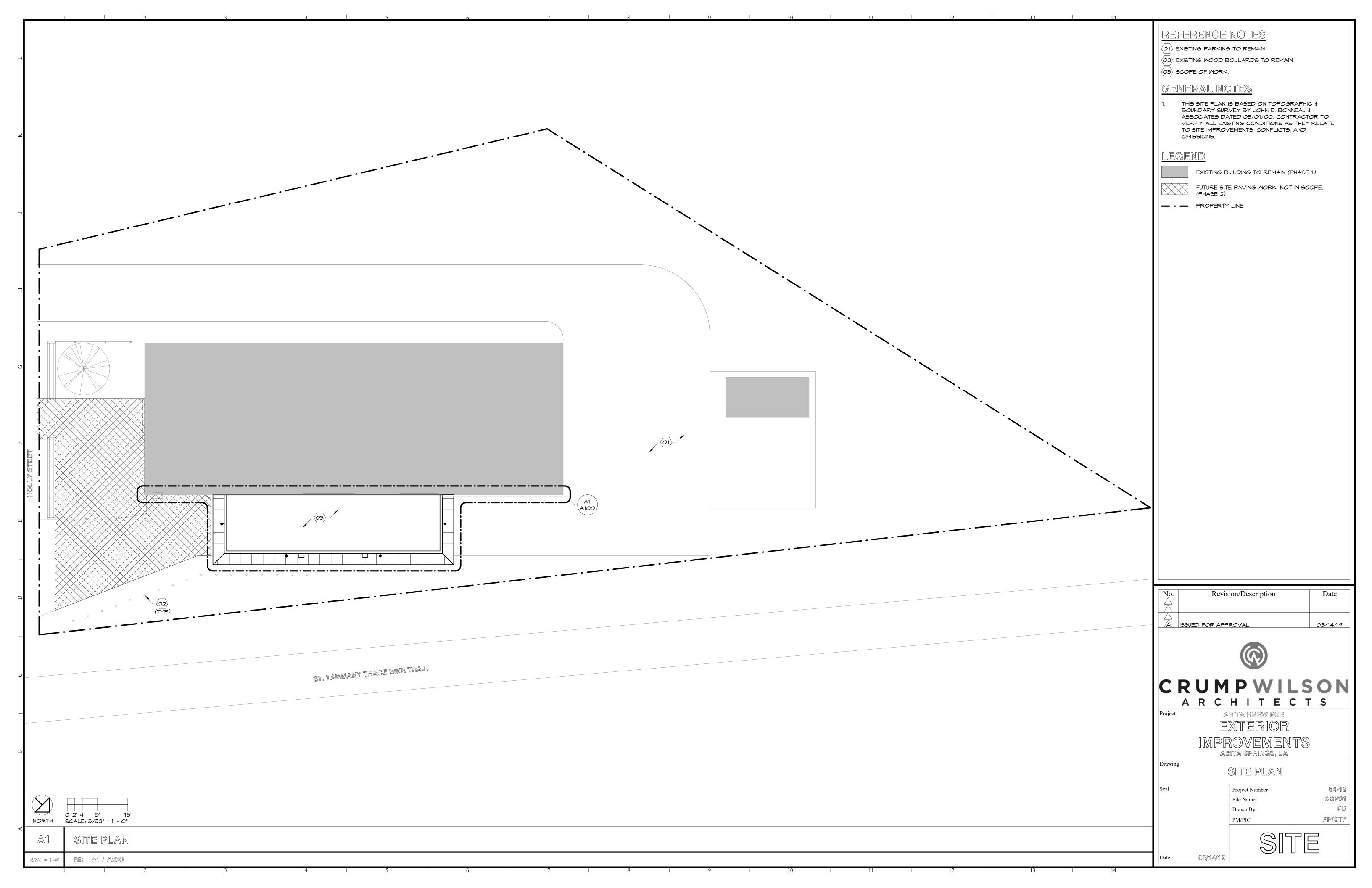
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File Name ABP01
Drawn By PD
PM/PIC PP/STF

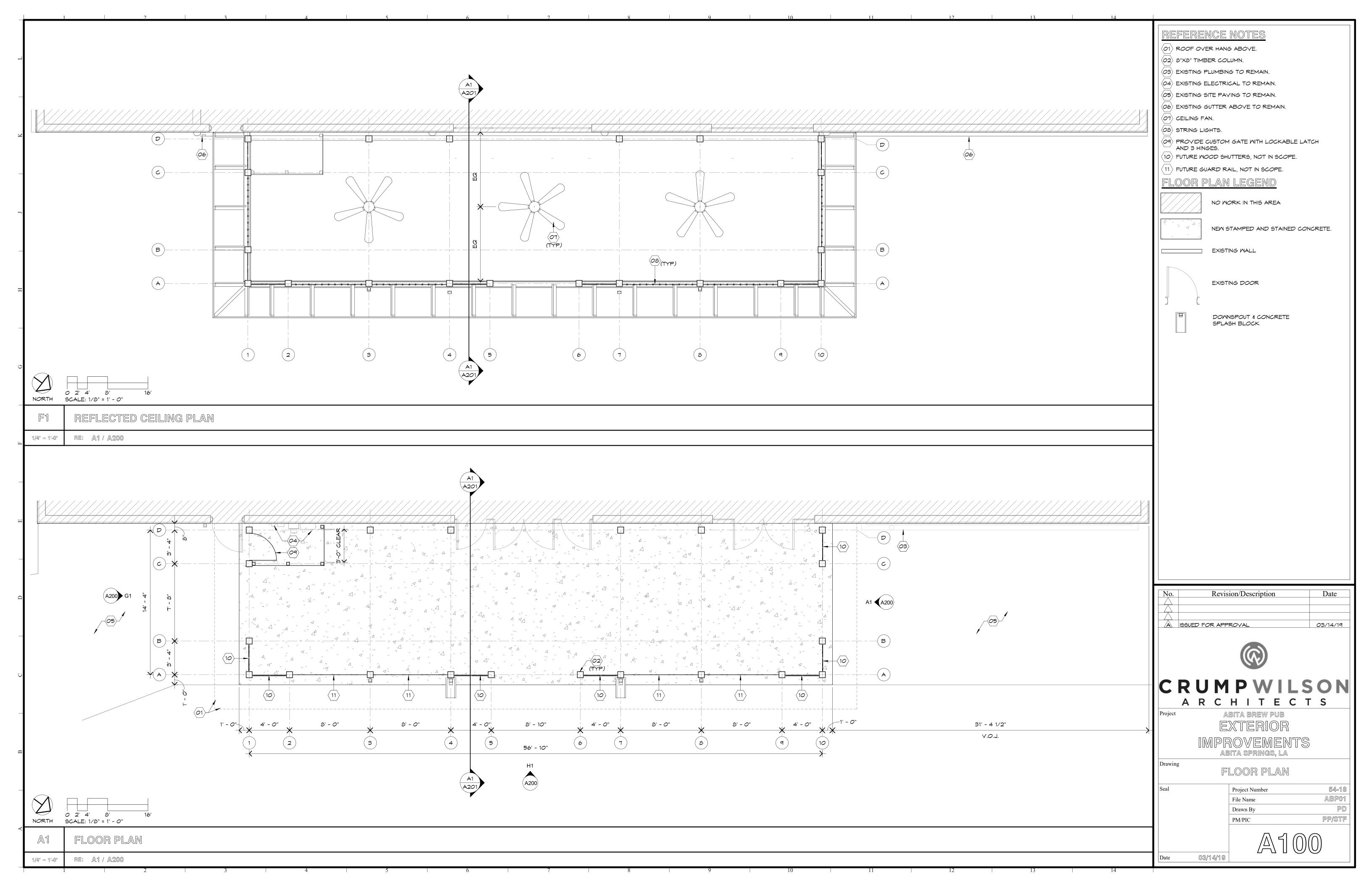
03/14/19 COVER

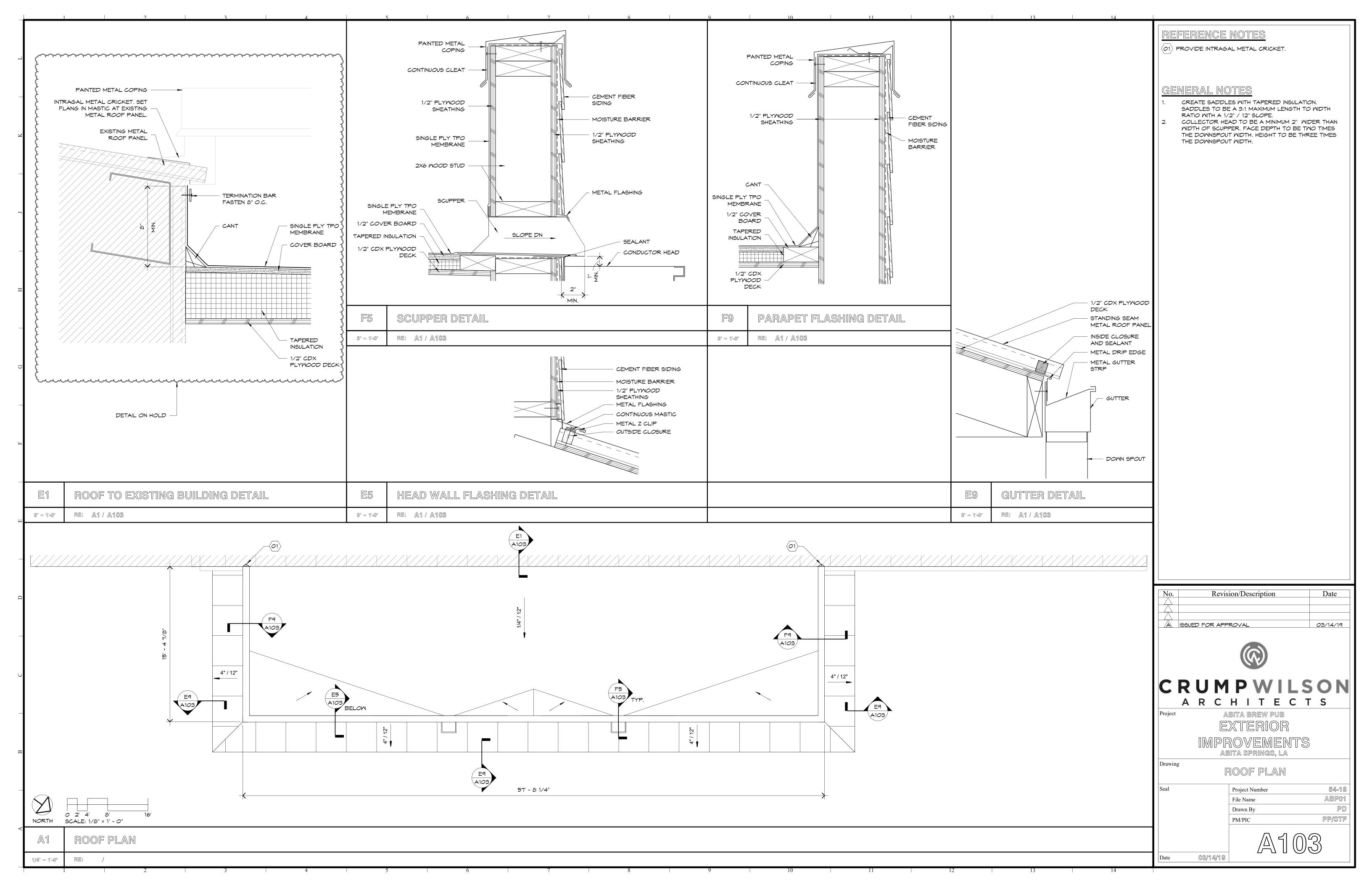
A11 ABBREVIATIONS & SYMBOLS

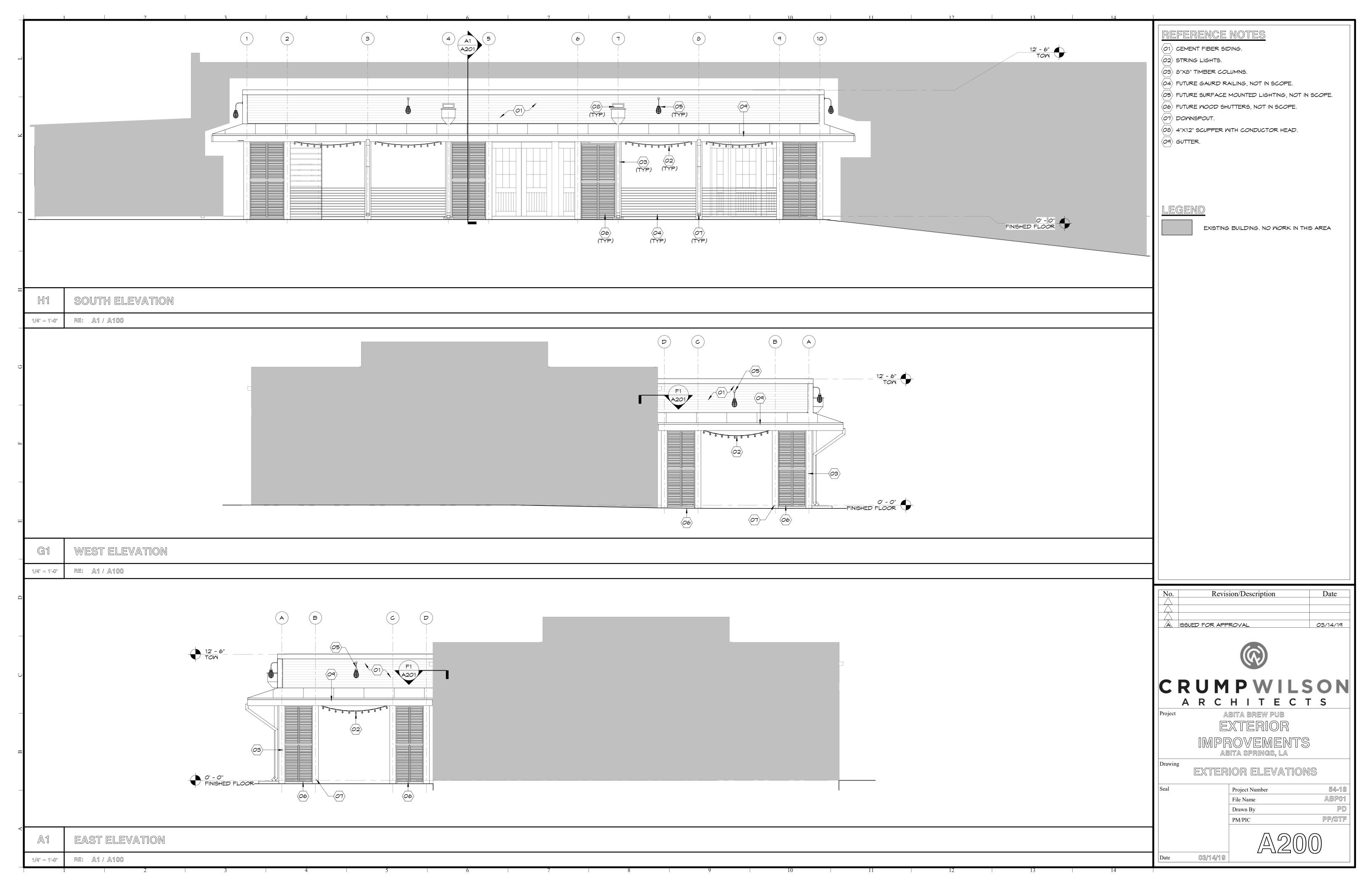
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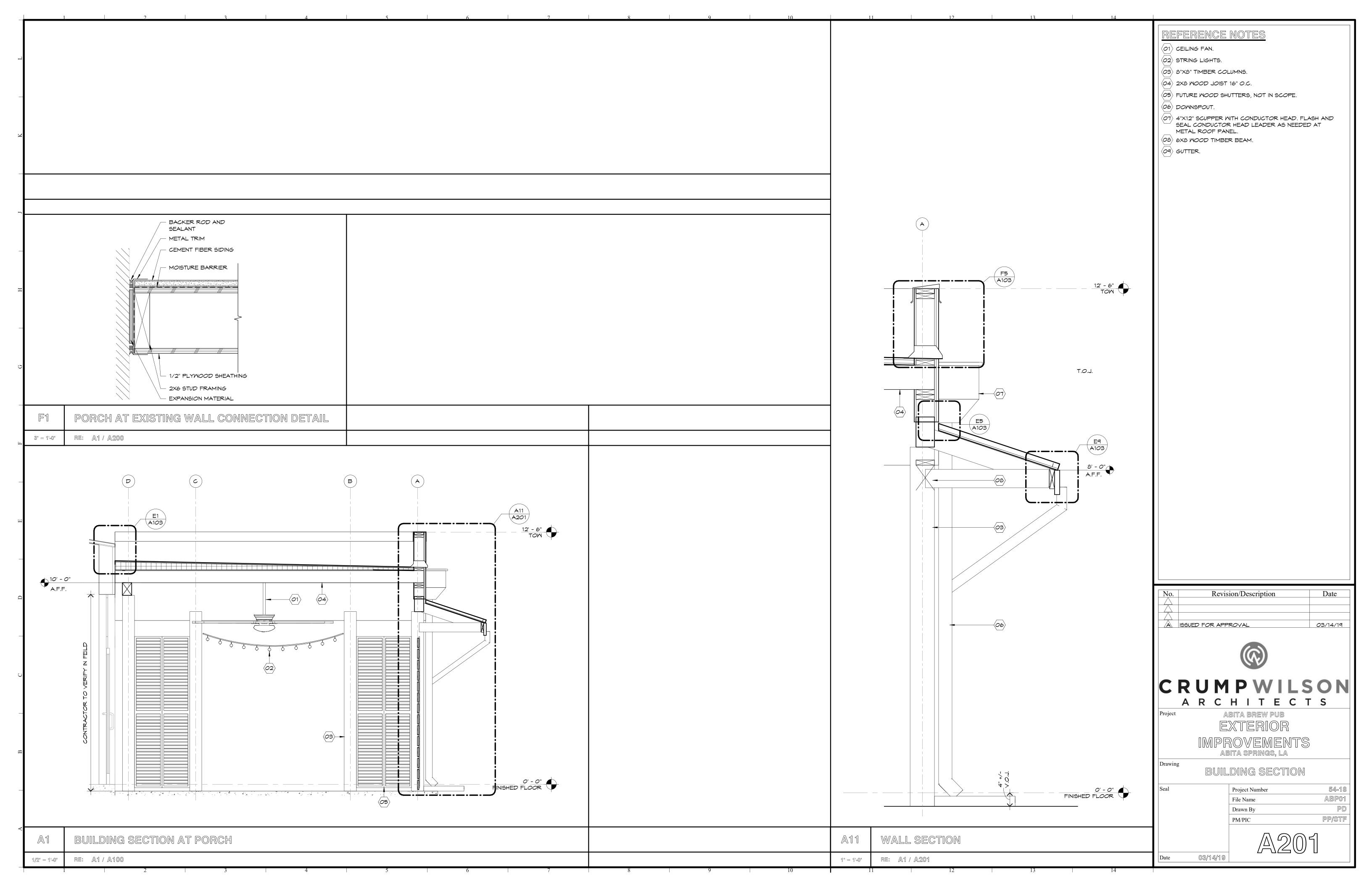












GENERAL NOTES

- ALL ELEVATIONS BASED ON FIRST FLOOR REFERENCE ELEVATION = 0'-0"
- COORDINATE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DETAILS WITH THESE DRAWINGS.

EXISTING CONDITIONS

CONTRACTOR NOTE CAREFULLY:

- 1. ALL DIMENSIONS OR ELEVATIONS TYING TO OR DEPENDENT UPON EXISTING STRUCTURE OR CONDITIONS SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO THE FABRICATION, ERECTION, OR CONSTRUCTION OF ANY ELEMENTS SO AFFECTED.
- IF ANY CONDITIONS ARE DIFFERENT FROM THAT INDICATED ON THE PLANS, CONTRACTOR SHALL NOTIFY ARCHITECT IMMEDIATELY AND APPROPRIATE DETAILS SHALL BE FURNISHED.
- ALL INFORMATION WILL BE SUBJECT TO VERIFICATION.
- EXISTING FOUNDATION ELEMENTS MAY OCCUR UNDER NEW WORK. VERIFY EXISTING CONDITIONS & NOTIFY ENGINEER IF CONDITIONS EXIST THAT REQUIRE ADJUSTMENT TO NEW WORK.

SITE PREP, EARTHWORK & FOUNDATION NOTES:

- FOOTINGS AND SLABS SHALL BEAR ON FIRM NATURAL UNDISTURBED SOIL OR COMPACTED FILL PLACED OVER FIRM NATURAL SOIL.
- STRIP ALL VEGETATION, TOPSOIL AND OTHER UNDESIRABLE MATERIAL PRIOR TO FILL PLACEMENT. THE EXPOSED SUBGRADE IN THE BUILDING, PARKING AND DRIVE AREAS SHALL BE PROOF-ROLLED. ANY SOILS WHICH ARE OBSERVED TO RUT OR DEFLECT EXCESSIVELY UNDER THE MOVING LOADS SHALL BE UNDERCUT AND REPLACED WITH PROPERLY COMPACTED SELECT FILL. AFTER PROOF-ROLLING, THE UPPER EIGHT (8) INCHES OF EXPOSED SOIL SHALL BE SCARIFIED AND THEN BE RE-COMPACTED TO AT LEAST 95 PERCENT OF THE STANDARD PROCTOR (ASTM D698) DENSITY. THE GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE SHALL WITNESS ALL PROOF-ROLLING AND UNDERCUTTING ACTIVITIES. THESE ACTIVITIES SHALL BE
- PERFORMED DURING A PERIOD OF DRY WEATHER. FILL MATERIAL SHALL BE A COHESIVE SOIL, FREE OF EXCESS SILT, WITH A PLASTICITY INDEX OF
- 15 TO 20. PLACE FILL IN 6" TO 8" LOOSE LIFTS AND COMPACT AT MOISTURE CONTENT WITHIN 2 PERCENT OF OPTIMUM MOISTURE TO A MINIMUM DRY DENSITY OF AT LEAST 95 PERCENT OF MAXIMUM AS OBTAINED IN THE STANDARD PROCTOR COMPACTION TEST (ASTM D698).
- KEEP FILL AREAS WELL DRAINED. PROTECT SLOPES FROM EROSION.

CONCRETE NOTES

- 1. ALL CONCRETE WORK TO BE IN ACCORDANCE WITH ACI 301 AND ALL RELATED ACI & ASTM REFERENCES CONTAINED THEREIN.
- 2. DEFORMED REINFORCING STEEL SHALL BE ASTM A615, GRADE 60 OR ASTM A616, GRADE 60.
- ALL CONCRETE IS NORMAL WEIGHT.
- 4. CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH AS FOLLOWS:
- 4.1. FOOTINGS, GRADE BEAMS = 3000 PSI MINIMUM
- 4.2. SLAB-ON-GRADE, ROOF SLAB = 4000 PSI (MAX W/C+P RATIO=0.45)
- 5. CEMENT CONFORMING TO THE FOLLOWING: 5.1. ASTM C150, TYPE I PORTLAND, GREY COLOR.
- 5.2. FLY ASH, ASTM C 618 CLASS C MAY BE USED AS A PARTIAL REPLACEMENT, NOT EXCEEDING 25% BY WEIGHT, FOR TYPE 1 CEMENT.
- 6. SLAB ON GRADE CONCRETE SHALL MEET ACI 301 TABLE 4.2.2.1 FOR MINIMUM CEMENTITIOUS
- MATERIAL CONTENT.
- 7. FINE AND COARSE AGGREGATES: ASTM C33. 8. WATER: CLEAN AND NOT DETRIMENTAL TO CONCRETE.
- 9. SUBMIT CONCRETE CURING METHOD FOR APPROVAL. ASTM C309 CURING COMPOUND IS ACCEPTABLE.
- 10. AIR ENTRAINMENT ADMIXTURE: ASTM C260.
- 11. SLUMP: 4" AT THE POINT OF DELIVERY, TOLERANCE ± 1 ".
- 12. USE MRWDA TO ACHIEVE $6"(\pm 1.5")$ SLUMP IN SLAB ON GRADE CONCRETE.
- 13. WATER REDUCING ADMIXTURE: ASTM C494 TYPE A; AND SAME MANUFACTURERS AS MID-RANGE
- 14. WATER REDUCING ADMIXTURE: ASTM C494 TYPE A; AND SAME MANUFACTURERS AS MID-RANGE WHEN USED TOGETHER.
- 15. DO NOT ADD WATER AT JOB SITE.
- 16. LAP SPLICE CONTINUOUS BARS 40 DIA. (15" MIN.) AT STAGGERED LOCATIONS.
- 17. PROVIDE CORNER BARS AT ALL CORNERS AND T-INTERSECTIONS OF GRADE BEAMS EQUAL TO HORIZONTAL REINFORCEMENT. EXTEND 40 DIA. EACH WAY FROM CORNERS.
- 18. PROVIDE (2)-#4 REINFORCEMENT BARS x 2'-0" AT RE-ENTRANT CORNERS AND AROUND RECTANGULAR HOLES IN SLABS UNLESS NOTED OTHERWISE. PLACE BARS DIAGONALLY TO CORNER WITH 1" CLEARANCE FROM THE TOP OF THE SLAB AT THE CORNER.
- 19. AT CONTRACTOR'S OPTION, FOOTING AND GRADE BEAM FACES NOT EXPOSED TO VIEW NEED NOT
- 20. REINFORCING STEEL DOWELED INTO EXISTING CONCRETE SHALL BE PLACED IN PROPERLY PREPARED DRILLED HOLES IN EPOXY PRODUCT TO ENGINEER APPROVAL.
- 21. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT; REINFORCING BARS SHALL BE PLACED AND TIED IN THE FORMS TO ACHIEVE CLEARANCES IN STRICT ACCORDANCE WITH THE REQUIREMENTS OF ACI-318, BUILDING CODE AND COMMENTARY (LATEST EDITION), AND SHALL MEET THE FOLLOWING CRITERIA:

CONCRETE CAST AGAINST & EXPOSED TO EARTH	3"
CONCRETE EXPOSED TO WEATHER (#6 & LARGER)	2"
CONCRETE EXPOSED TO WEATHER (#5 & SMALLER)	1½"
CONCRETE NOT EXPOSED TO WEATHER (#11 & SMALLER)	1"

- 22. SOME AMOUNT OF MINOR CRACKING IS TO BE EXPECTED IN ANY CONCRETE WORK. CONCRETE SHRINKAGE WILL ALWAYS CAUSE SOME AMOUNT OF CRACKING. REINFORCING STEEL HAS BEEN DESIGNED AND DRAWINGS SHOW THE PLACEMENT TO LIMIT ANY CRACK WIDTHS TO AN ACCEPTABLE WIDTH. THE DESIGNER IS NOT RESPONSIBLE FOR MINOR CRACKING IN THE CONCRETE WORK THAT DOES NOT AFFECT THE STRENGTH OR SERVICEABILITY OF THE STRUCTURE OF WHICH THE CONCRETE IS A PART.
- 23. SAMPLING AND TESTING FOR QUALITY CONTROL DURING CONCRETE PLACEMENT SHALL BE PER "COMPRESSIVE STRENGTH TESTS" ASTM C 39; ONE SET FOR EACH DAY'S POUR EXCEEDING 5 YD. PLUS ADDITIONAL SETS FOR EACH 50 CU. YD. MORE THAN THE FIRST 25 CU. YD. OF EACH CONCRETE CLASS PLACED IN ANY ONE DAY; ONE SPECIMEN TESTED AT 7 DAYS, TWO SPECIMENS TESTED AT 28 DAYS, AND ONE SPECIMEN RETAINED IN RESERVE FOR LATER TESTING IF REQUIRED.

CONSTRUCTION DOCUMENTS REQUIRED ENGINEERING DESIGN INFORMATION

PROJECT: Abita Brew Pub Addition

	IBC 201	5 LOADING INFOR	MATION
1603.1.1 Floor Live Load	1st floor	250 psf	Live Load reduction is not used in floor loading
	Upper floors	N/A	
	Stairs, lobbies, corridors	100 psf	
	Corridors above 1st flr	N/A	
	Concentrated anywhere	2000 lbs	
1603.1.2 Roof Live Load		20 psf	Live Load reduction is not used in roof loading
1603.1.3 Roof Snow Load		0 psf	The ground snow load, Pg, shall be indicated
1603.1.4 Wind Design Data	$V_{ultimate}$ =	133 mph	1 Ultimate design wind speed (3-second gust)
	V_{asd} =	105 mph	1 Nominal design wind speed
	Risk Category:	II	2
	Exposure	С	3 Wind exposure Category
	Enclosed Bldg	0.18	4 Internal pressure coefficient
	On drawings		5 Components and cladding wind pressures
1603.1.5 Earthquake Design Data	Risk Category:	II	1
		1.0	2 Seismic importance factor, l _{e'}
	S _s =	0.102	3 Mapped spectral response accelerations,
	S ₁ =	0.057	S_S and S_1
		Е	4 Site class.
	S _{ds} =	0.171	5 Spectral response coefficients, S _{DS1} , and S _{D1}
	S _{d1} =	0.133	
		С	6 Seismic design category,
	W	ood Moment Frame	7 Basic seismic-force-resisting system(s).
		1.4 k, LRFD	8 Design base shear.
		0.114	9 Seismic response coefficient(s), C _S
		1.5	10 Response modification factor(s), R.
	Equ	uivalent Lateral Force	11 Analysis procedure used
1603.1.6 Geotechnical Information		1500 psf	Allowable bearing pressure used in design

WOOD FRAMING NOTES

- 1. WOOD FRAMING SHALL BE SOUTHERN PINE, NO. 2 K.D. (15% MAX. MOISTURE CONTENT) OR EQUIVALENT. MINIMUM ALLOWABLE BENDING STRESS SHALL BE 1,300 PSI.
- 2. STRUCTURAL GLUED LAMINATED TIMBER SHALL BE PRODUCED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC). MINIMUM ALLOWABLE BENDING STRESS
- SHALL BE 2,400 PSI (DRY CONDITIONS) CONNECTIONS FOR STRUCTURAL TIMBER SHALL BE GALVANIZED STRONG-TIE CONNECTORS BY THE SIMPSON COMPANY OR APPROVED EQUAL.
- 4. WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE FOUNDATION GRADE
- PRESSURE-TREATED SOUTHERN PINE. USE GALVANIZED NAILS IN PRESSURE-TREATED WOOD. PLYWOOD DIAPHRAGMS SHALL APA STRUCTURAL I RATED PLYWOOD WITH THICKNESS GIVEN HEREIN OR AS NOTED IN THE STRUCTURAL AND/OR ARCHITECTURAL DOCUMENTS.
- 6. USE ONLY ADHESIVES CONFORMING TO PERFORMANCE SPECIFICATION AFG-01 OR ASTM D3498 (CHECK FLOOR PANEL ADHESIVE LABEL FOR COMPLIANCE).

NAILING SCHEDULE

ROOF DI NAILING		
LOCATION	SIZE	SPACING
BOUNDARY	8d	6"
PANEL EDGE	8d	6"
FIELD	8d	6"

1. MINIMUM PENETRATION IN FRAMING IS 11/2".

2. DIAPHRAGMS ARE UNBLOCKED.

a	Effective					Pressure	e (psf)		
Roof	Area (ft²)	Zon	e 1	Zon	e 2	Zon	e 3	OVERHANG Zone 2/0H	OVERHANG Zone 3/OH
3 2 3		Positive	Uplift	Positive	Uplift	Positive	Uplift	Uplift	Uplift
	10	16	-39	16	-65	16	-97	-41	-61
Clade	20	16	-38	16	-58	16	-81	-36	-48
(5/0H) (2/0H) (5/0H) E E	50	16	-36	16	-49	16	-59	-36	-31
ROOF PLAN	100	16	-35	16	-42	16	-42	-35	-17
JOIST BRIDGING IS NOT SHOWN ON PLANS, PROVIDE & INSTALL AS	STRENGTH	LEVEL (JLTIMATE	E) COMPO	ONENT &	: CLADDIN	NG LOAD	ING ON ROC	F AREAS:
REQ'D BY JOIST MFG. FOR SPAN & UPLIFT REQUIREMENTS GIVEN HERE	ALL RO	OF COMP			3LE (a=	4 FT)		ESSURES SH	OWN IN

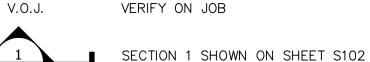
	F.(():		Pressur	e (psf)		a
Walls	Effective Area (ft²)	Zon	e 4	Zon	ne 5	
	/ 50 (/	Positive	Negative	Positive	Negative	5 4 5
s D	10	39	-42	39	-52	(5) (4) (5)
nent	20	37	-40	37	-48	WALLS
Components and Cladding	50	35	-38	35	-44	COMPONENT AND CLADDING PRESSURES (PSF) FROM DESIGN WIND ON WALL AREAS (a=4 FT
- 0	100	33	-36	33	-40	(5.00)

LEGEND

U.N.O.

REFERENCE ELEVATION

FINISHED FLOOR ELEVATION



UNLESS NOTED OTHERWISE



Revision/Description

CR	UMPWILSON RCHITECTS
Project	ABITA BREW PUB EXTERIOR

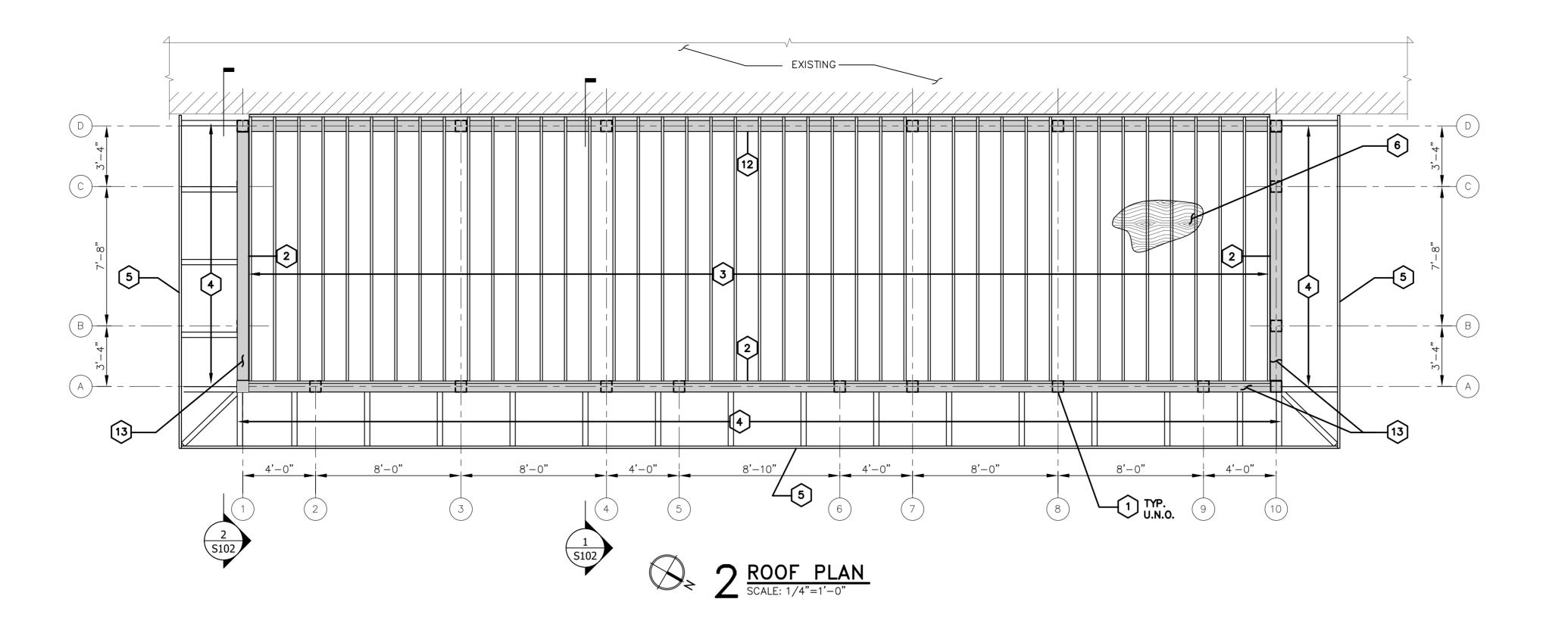
GENERAL NOTES

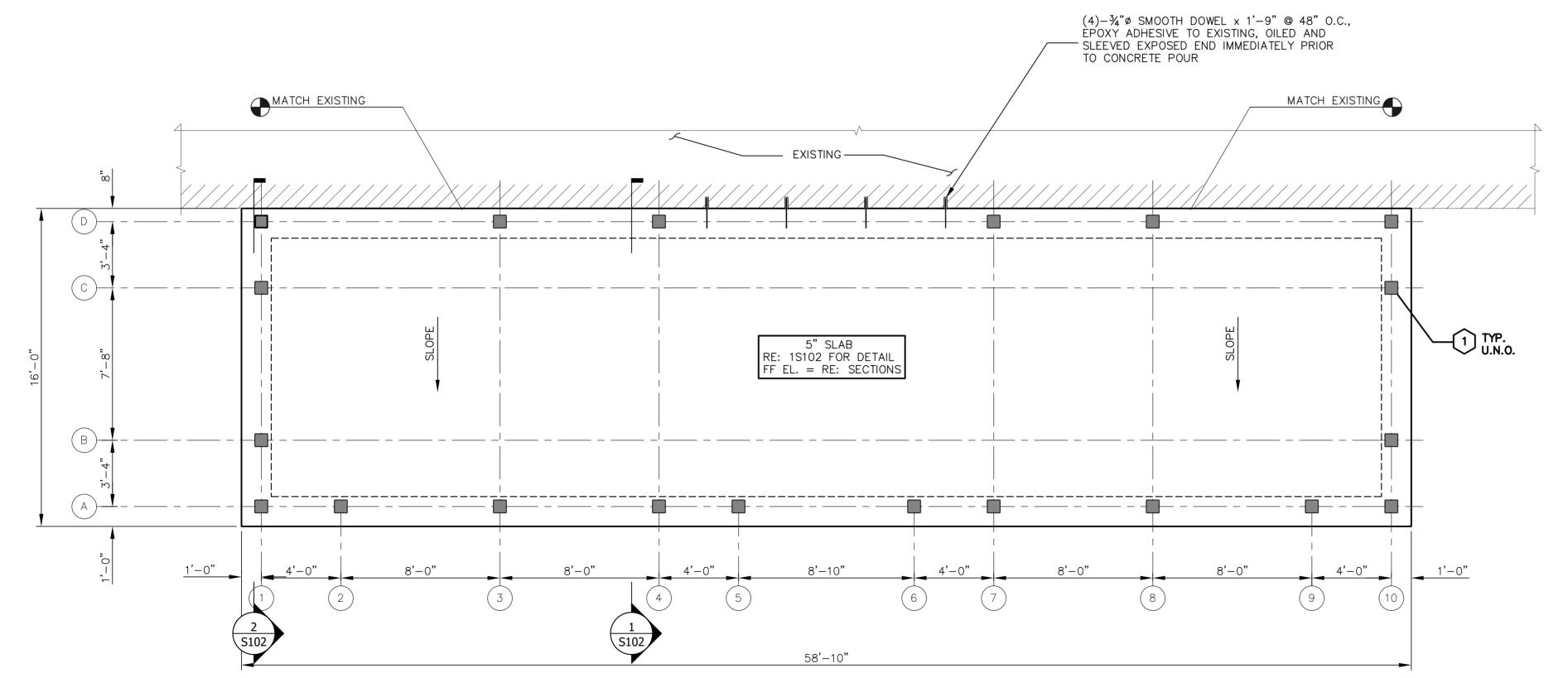
ABITA SPRINGS, LA

54-18 Project Number AB0í File Name PHH Drawn By TMW PM/PIC

03/11/19

Date





T FOUNDATION PLAN

SCALE: 1/4"=1'-0"

RE	EFERENCE NOTES
MARK	NOTES
1	#2 SYP 8x8 TIMBER COLUMN
2	#2 SYP 8x8 TIMBER BEAM
3	#2 SYP 2x8 ROOF JOISTS @ 16" O.C.
4	EAVE FRAMING, RE: ARCH
5	#2 SYP 2x6 CONT, VERIFY W/ ARCH
6	PLYWOOD ROOF DECK, SEE NOTE A
7	2x6 @ 16" O.C.
8	2x6
9	SIMPSON CPS7 BASE, RE: 4S102
10	#2 SYP 8x8 BRACE
11)	SIMPSON HL46PC HEAVY ANGLE
12	#2 SYP 8x12 TIMBER BEAM
13	PARAPET ABOVE, RE: SECTIONS
14	SIMPSON PS218PC STRAP EA SIDE
15	SIMPSON HURRICANE CLIP

NOTE A

PROVIDE APA-PERFORMANCE-RATED PANELS COMPLYING WITH REQUIRED GRADE, 24" MINIMUM ROOF SPAN RATING, EXPOSURE 1 BOND CLASSIFICATION AND EDGE DETAILS PER ARCH.

- MINIMUM THICKNESS: ¹⁵/₃₂", VERIFY W/ ARCH.
 SPAN RATINGS: PROVIDE PANELS WITH SPAN RATINGS REQUIRED TO MEET "CODE PLUS" PROVISIONS OF APA FORM NO. E30, "APA DESIGN/CONSTRUCTION GUIDE: RESIDENTIAL & COMMERCIAL."
- 3. CONNECT ROOF SHEATHING W/ 8d NAILS AT 6" O.C. AT PANEL EDGES AND 12" O.C. INTERIOR SUPPORTS.



No.	Revision/Description	Date
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CRUMPWILSON ARCHITECTS

Troject

abita brew pub EXTERIOR

IMPROVEMENT abita springs, la

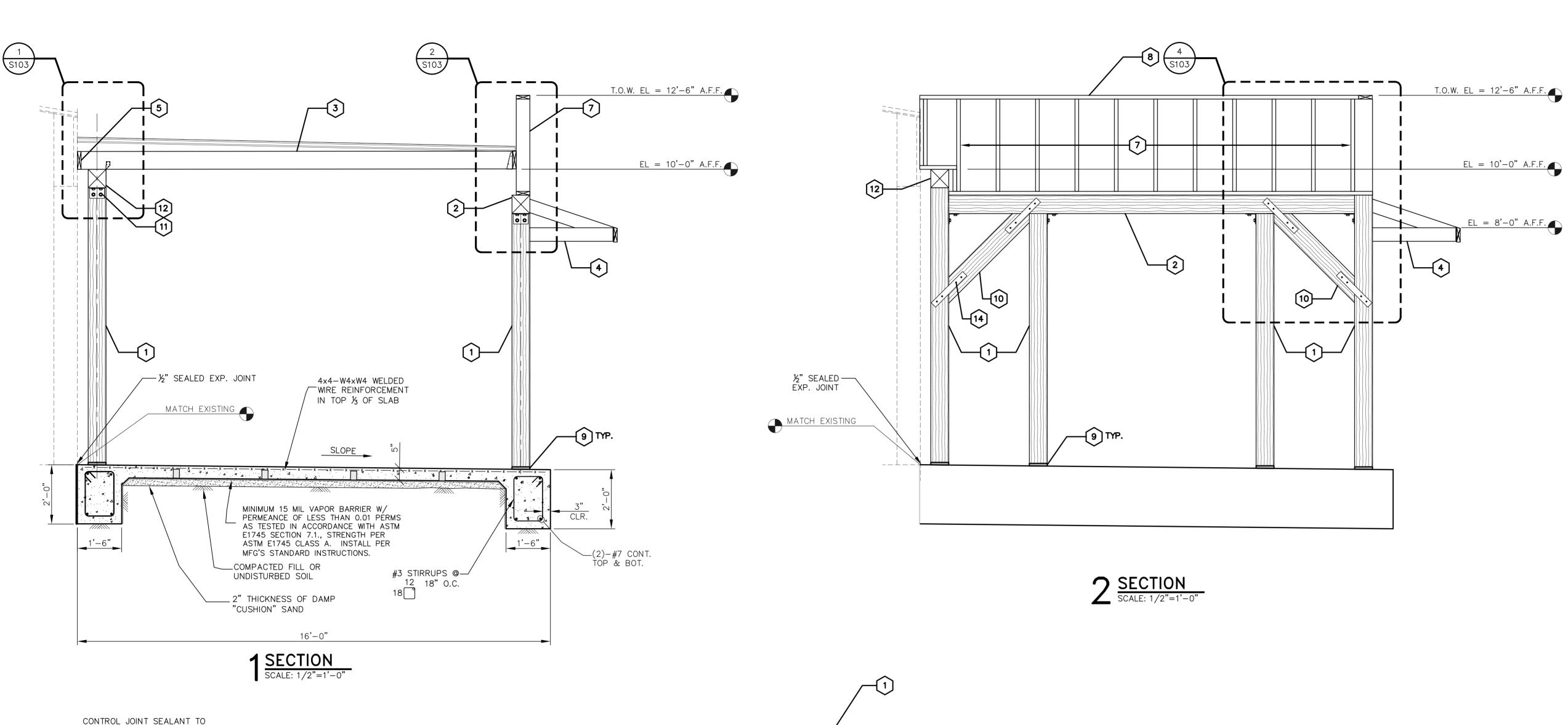
Drawing

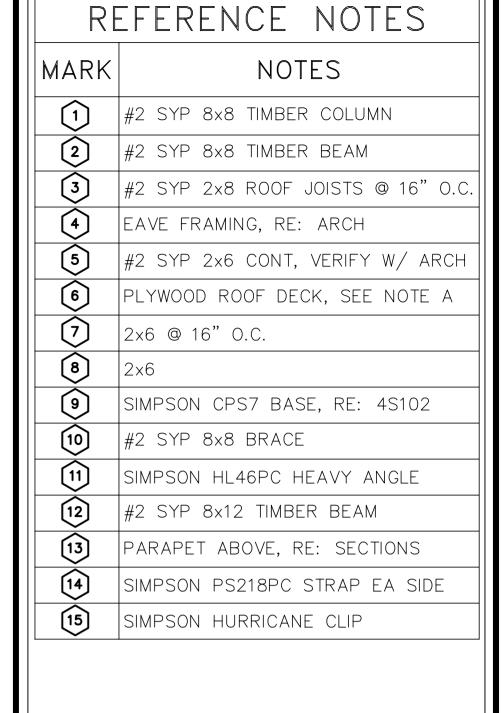
STRUCTURAL PLANS

Project Number 54-18
File Name AB01
Drawn By PHH
PM/PIC TMW

\$101

03/11/19







Revision/Description Date

(225) 343-4129

PROJECT No. 90 2019005



CRUMPWILSON ARCHITECTS

ABITA BREW PUB

ABITA SPRINGS, LA

Building sections & details

Project Number File Name Drawn By PM/PIC

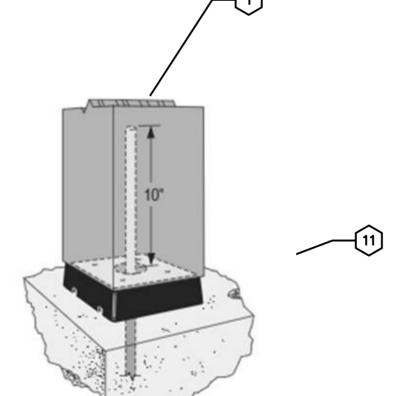
03/11/19

54-18

AB01

PHH

TMW



POST BASE DETAIL

INSTALLATION:

• DRILL A 34" Ø HOLE, 10" INTO THE CENTER OF THE POST.

 CLEAN OUT DUST. • FILL HOLE HALFWAY WITH SIMPSON STRONG-TIE SET-3G EPOXY ANCHORING

ADHESIVE. • INSERT ALL-THREAD ROD AND ALLOW EPOXY TO SET AND CURE.

SECURE STANDOFF TO POST USING FOUR 0.148" X 3" NAILS EXCEPT PBV WHICH

USES FOUR STRONG-DRIVE SDS HEAVY-DUTY CONNECTOR SCREWS.

CONCRETE:

• DRILL A ¾"Ø HOLE PER ANCHOR DESIGN.

CLEAN OUT DUST.

• FILL HOLE HALFWAY WITH SIMPSON STRONG-TIE SET-3G EPOXY ANCHORING

• INSERT POST SUBASSEMBLY INTO HOLE AND ALLOW EPOXY TO SET AND CURE.

3 SLAB CONTROL JOINT SCALE: 3/4" = 1'-0" 1. BEGIN SAWCUTTING JOINTS WHEN THE CONCRETE IS FIRM ENOUGH NOT TO BE TORN OR DAMAGED BY THE BLADE. 2. JOINTS PRODUCED USING CONVENTIONAL PROCESSES ARE MADE WITHIN 4 TO 12 HOURS AFTER THE SLAB HAS BEEN FINISHED IN AN AREA (4 HOURS IN HOT WEATHER TO 12 HOURS IN COLD WEATHER). FOR EARLY-ENTRY DRY-CUT SAWS, THE WAITING PERIOD SHOULD VARY FROM 1 HOUR IN HOT WEATHER TO 4 HOURS IN COLD WEATHER AFTER COMPLETING THE FINISHING OF THE SLAB IN THAT JOINT LOCATION.

3. MARK THE EXACT LOCATION OF THE CUT WITH A CHALK LINE OR STRING LINE

SO THE SAW OPERATOR KNOWS WHERE TO SAW THE JOINT.

4. BEFORE INSTALLATION OF JOINT FILLER, CLEAN PER MFG. DIRECTIONS.

RE: 1S102 FOR

SLAB ON GRADE

5/16"♥ CLOSED CELL

POLYETHYLENE BACKER ROD

JOINTS REQUIRED EVERY 12 FT MAX O.C., EXACT JOINT LAYOUT WILL BE COORDINATED PRIOR TO CONSTRUCTION.

MEET FEDERAL SPECIFICATION

TT-S-00230C AND ASTM C920.

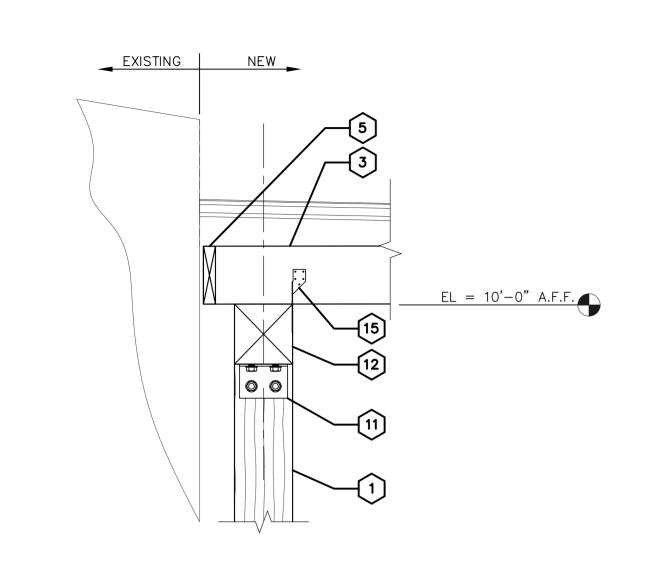
SUBMIT PRODUCT TO ENGINEER FOR REVIEW. CLOSELY FOLLOW

ALL MFG. DIRECTIONS

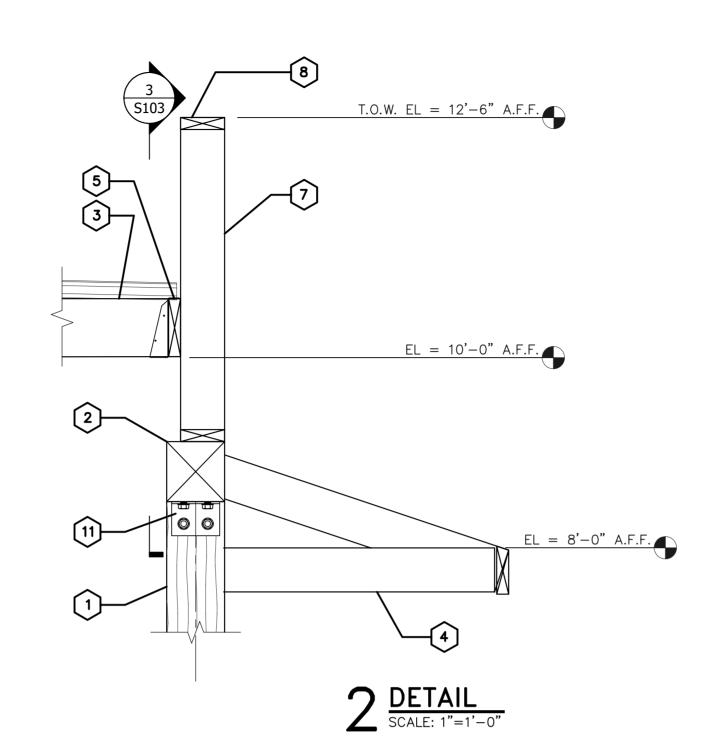
NEW CONC. POUR

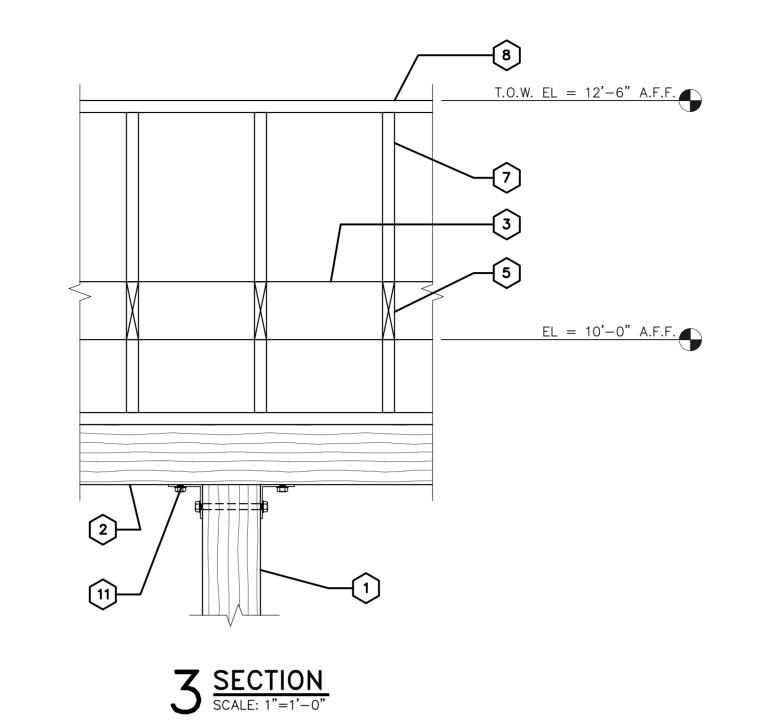
CUT EVERY OTHER

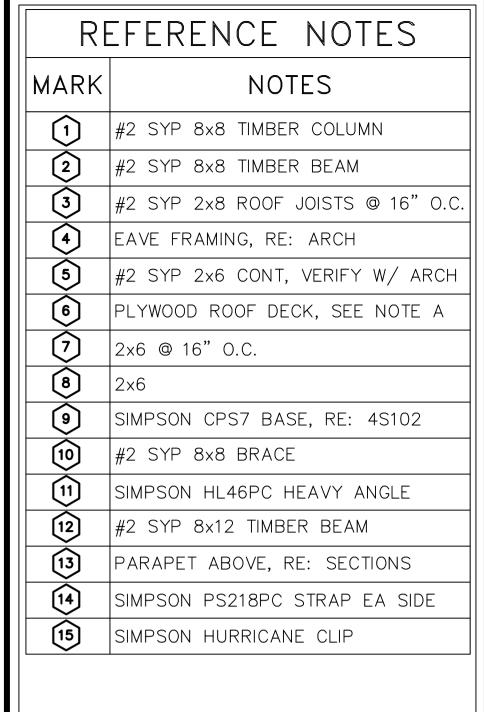
WIRE @ JOINTS



1 DETAIL
SCALE: 1"=1'-0"









No.	Revision/Description	Date
	- *	



CRUMPWILSON ARCHITECTS

Project

ABITA BREW PUB

IMPROVEMENTS abita springs, la

Drawing

SECTION & DETAILS

Seal

03/11/19

File Name AB01
Drawn By PHH
PM/PIC TMW

\$103

54-18

